

Problematic Clay Soils Modification Using Bush mango Fibre as Stabilizer

Charles Kennedy¹, Gbinu Samuel Kabari², Letam Leelee Prince³

¹Faculty of Engineering, Department of Civil Engineering, Rivers State University, Nkpolu, Port Harcourt, Nigeria

^{2,3}School of Engineering, Department of Civil Engineering, Kenule Beeson Saro-Wiwa Polytechnic, Bori, Rivers State, Nigeria.

Authors E-mail: ¹ken_charl@yahoo.co.uk, ²kabarisamuel@gmail.com, ³leeprices076@yahoo.com

Abstract: Majority of the roads within the Niger Delta regions of Nigeria constructed on black cotton soils and are prone to severe cracks, degradation and differential settlement that has led to constant rehabilitation seasonally. The soils fell below standard for its application as road constructional materials. The study experimented on the use of *irvingia gabonensis* fibre (Bush Mango) to stabilize clay soils with percentage ratios of 0.25%, 0.50%, 0.75% and 1.0%. Preliminary investigations revealed the soils as A – 7 – 6 /CH on the AASHTO classification schemes / Unified Soil Classification System. The soils were dark grey in colors (from wet to dry states) with plastic index of 31.10%, 24.55%, 31.05%, and 32.17% respectively for Iwofe, Chokocho, Ndoni, and Ogbele Roads. The soils has unsoaked CBR values of 7.35%, 7.75%, 8.15%, and 7.85% and soaked CBR values of 6.35%, 6.23%, 7.05% and 5.55%. Unconfined compressive strength (UCS) values of 87.85kPa, 78.75kPa, 105.75kPa and 85.35kPa. Results of compaction test determination showed decreased in MDD values while OMC values increased, both parameters adhered to corresponding percentage increase ratios. Comparably, results indicated increased in CBR altered soils with optimum mixed percentage ratios of 0.7% to soils. Similarly, results of UCS stabilized soils increased with percentages ratio variations. Results showed low values of plastic index with corresponding percentage increased ratios. Entire results showed the potential use of *irvingia gabonensis* fibre for expansive soils treatment.

Keywords: Clay soils, *Irvingia Gabonensis* Fibre, CBR, UCS, Consistency, Compaction.

1. INTRODUCTION

Soil stabilization depends mainly on chemical reactions between stabilizer and soil minerals to achieve the desired effect. Generally, the additions of additives such as fibres to expansive soils increase the strength, bearing capacity and durability of the soil. Natural fibres such as hey, wood and bamboo have been used for the improvement of construction materials (Khedari *et al.* [1]), the use of appropriate elements in soil improves its engineering properties such as strength, hardness and deformability. Currently, natural fibres such as kenaf, coir, banana, jute, flax, sisal, palm, reed, bamboo and wood fibres are used for soil reinforcement and stabilisation (Ramakrishna and Sundararajan [2]).

Charles *et al.* [3] investigated the effectiveness of natural fibre, *costus afer* bagasse (Bush sugarcane bagasse fibre (BSBF) as soil stabilizer / reinforcement in clay and lateritic soils with fibre inclusion of 0.25%, 0.50%, 0.75% and 1.0%. They concluded that both soils decreased in MDD and OMC with inclusion of fibre percentage, CRB values increased tremendously with optimum values percentage inclusion at 0.75%, beyond this value, crack was formed which resulted to potential failure state.

Ghavami *et al.* [4] observed that the addition of 4 % coconut and sisal fibres to soil causes its deformability to increase significantly. Besides, the creation of cracks in dry seasons was highly lessened.

Prabakar and Sridhar [5] studied on soil specimens reinforced with sisal fibres showed that both fibre content and aspect ratio have important influences in shear strength parameters (c , ϕ).

Bouhicha *et al.* [6] used the shear box test method to evaluate the strength of compacted earth reinforced with barley straw. Their work was part of a wider study of the physical and mechanical properties of fibre-reinforced compressed earth blocks. Their test results are showed that a 1.5 and 3.5 % (by weight of soil) addition of straw increased the apparent cohesion by up to 50 % (from 330 to 493kPa), but decreased the angle of internal friction.

2. MATERIALS AND METHODS

2.1 Materials

2.1.1 Soil

The soils used for the study were collected within failed sections of the at 1.5 m depth from Iwofe Town Road, in Obio/Akpor Local Government Area, Chokocho Town Road, in Etche Local Government Area, Ndoni Town Road, in Ogba/Egbema/Ndoni Local Government Area and Ogbele Town Road in ahoada – East Local Government Area, all in Rivers State, Nigeria.

2.1.2 Irvinga Gabonensis Fibre (Bush Mango)

The irvinga gabonensis, popularly called Bush mango, with Nigerian native name (Egbono) are widely spread plants across Nigerian bushes and farm land with edible fruits that bears the fibre , they are collected from at Olokuma village, a river side area in Ubie Clan, Ahoada-West, Rivers State, Nigeria.

2.2 Method

2.2.1 Sampling Locality

The soil sample used in this study were collected along Iwofe Town, (latitude 4.49° 41'S and longitude 6.57° 24'E), Chokocho Town, (latitude 4.9882° N ° 34'S and longitude 7.0525° ° 13'E), Ndoni Town, latitude 5.5487 ° 21'S and longitude 6.5917° ° 39'E), Ogbele Town, (latitude 4.9198 ° 23'S and longitude 6.6751 ° 34'E) all in Rivers State, Nigeria.

2.2.2 Test Conducted

Test conducted were (1) Moisture Content Determination (2) Consistency limits test (3) Particle size distribution (sieve analysis) and (4) Standard Proctor Compaction test, California Bearing Ratio test (CBR) and Unconfined compressive strength (UCS) tests;

2.2.3 Moisture Content Determination

The natural moisture content of the soil as obtained from the site was determined in accordance with BS 1377 (1990) Part 2. The sample as freshly collected was crumbled and placed loosely in the containers and the containers with the samples were weighed together to the nearest 0.01g.

2.2.4 Grain Size Analysis (Sieve Analysis)

This test is performed to determine the percentage of different grain sizes contained within a soil. The mechanical or sieve analysis is performed to determine the distribution of the coarser, larger-sized particles.

2.2.5 Consistency Limits

The liquid limit (LL) is arbitrarily defined as the water content, in percent, at which a part of soil in a standard cup and cut by a groove of standard dimensions will flow together at the base of the groove for a distance of 13 mm (1/2in.) when subjected to 25 shocks from the cup being dropped 10 mm in a standard liquid limit apparatus operated at a rate of two shocks per second.

2.2.6 Moisture – Density (Compaction) Test

This laboratory test is performed to determine the relationship between the moisture content and the dry density of a soil for a specified compactive effort.

2.2.7 Unconfined Compression (UC) Test

The unconfined compressive strength is taken as the maximum load attained per unit area, or the load per unit area at 15% axial strain, whichever occurs first during the performance of a test. The primary purpose of this test is to determine the unconfined compressive strength, which is then used to calculate the unconsolidated undrained shear strength of the clay under unconfined conditions

2.2.8 California Bearing Ratio (CBR) Test

The California Bearing Ratio (CBR) test was developed by the California Division of Highways as a method of relegating and evaluating soil- subgrade and base course materials for flexible pavements.

3. RESULTS AND DISCUSSIONS

Preliminary results on clay soils as seen in detailed test results given in Tables: 3.1 showed that the physical and engineering properties fall below the minimum requirement for such application and needs stabilization to improve its properties. The soils classified as A – 7 – 6 /CH on the AASHTO classification schemes / Unified Soil Classification System as shown in table 3.1 and are less matured in the soils vertical profile and probably much more sensitive to all forms of manipulation that other deltaic lateritic soils are known for (Ola [7]; Allam and Sridharan [8]; Omotosho and Akinmusuru [9]; Omotosho [10]). The soils are dark grey in colour (from wet to dry states) plastic index of 31.10%, 24.55%, 31.05%, and 32.17% respectively for Iwofe, Chokocho, Ndoni, and Ogbele Town Roads. The soil has unsoaked CBR values of 7.35%, 7.75%, 8.15%, and 7.85% and soaked CBR values of 6.35%, 6.23%, 7.05% and 5.55%, unconfined compressive strength (UCS) values of 87.85kPa , 78.75kPa, 105.75kPa , and 85.35kPa when compacted with British Standard light (BSL), respectively.

3.1 Compaction Test Results

Natural clay soils compaction test results at 100% are maximum dry density (MDD) 1.685KN/m³, 1.635KN/m³, 1.657KN/m³, 1.697KN/m³ and optimum moisture content (OMC) are 15.28%, 16.28%, 16.05% and 15.73%. In contrast, stabilized clay soils of Iwofe, Chokocho, Ndoni and Ogbele roads with 0.25%, 0.5%, 0.75 and 1.0% to clay soils percentage ratios maximum values are (MDD) 1.550KN/m³ 1.493KN/m³ 1.518KN/m³ 1.542KN/m³ and (OMC) 16.60%, 17.55%, 17.28% and 17.05% . Results of compaction test determination showed decreased in MDD values while OMC values increased, both parameters adhered to corresponding percentage increase ratios.

3.2 California Bearing Ratio (CBR) Test

Clay soils at 100% results of CBR values soils has unsoaked 7.35%, 7.75%, 8.15%, 7.85% and soaked 6.35%, 6.23%, 7.05% and 5.55%. Altered clay soils with Irvinga Gabonensis fibre with 0.25%, 0.5%, 0.75% and 1.0% values at maximum are unsoaked 15.25 %, 15.35%, 15.35%, 15.85%, and unsoaked 14.80%, 13.65%, 14.83% and 13.85%. Comparably, results indicated increased in CBR altered soils with optimum mixed percentage ratios of 0.7% to soils.

3.3 Unconfined Compressive Strength Test

Results obtained of clay soils at preliminary tests are 87.85kPa, 78.75kPa, 105.75kPa and 85.35kPa. Fibre treated soil results maximum values are 286kPa, 245kPa, 295kPa and 274kPa. Comparably, results of UCS stabilized soils increased with percentages ratio variations.

3.4 Consistency Limits Test

Results of consistency limits (Plastic index) at 100% soils are 31.10%, 24.55%, 31.05%, and 32.17%. Fibre stabilized clay soils yielded 28.93%, 23.25%, 28.85% and 29.93%. Results showed low values of plastic index with corresponding percentage increased ratios.

Table 3.1: Engineering Properties of Soil Samples

Location Description	Iwofe Road Obio/Akpor L.G.A	Chokocho Road Etche L.G.A	Ndoni Road Ogba/Egbema/ Ndoni L.G.A	Ogbele Road Ahoda East L.G.A
Depth of sampling (m)	1.2	1.2	1.2	1.2
Percentage(%) passing BS sieve #200	76.35	80.25	83.65	78.25
Colour	greenish	Greenish	greenish	greenish
Specific gravity	2.52	2.58	2.45	2.44
Natural moisture content (%)	42.58	48.35	44.65	44.30
Consistency Limits				
Liquid limit (%)	68.35	53.85	62.40	58.75
Plastic limit (%)	37.25	29.30	31.35	26.58
Plasticity Index	31.10	24.55	31.05	32.17
AASHTO soil classification	A-7-6	A-7-6	A-7-6	A-7-6
Unified Soil Classification System	CH	CH	CH	CH
Optimum moisture content (%)	15.28	16.28	16.05	15.73
Maximum dry density (kN/m ³)	1.685	1.635	1.657	1.697

Compaction Characteristics				
Gravel (%)	0.0	0	0	0
Sand (%)	13.18	12.3	12.8	16.5
Silt (%)	42.3	48.5	42.3	48.2
Clay (%)	44.6	38.2	44.9	35.3
Unconfined compressive strength (kPa)	87.85	78.75	105.75	85.35
California Bearing Capacity (CBR)				
Unsoaked (%) CBR	7.35	7.75	8.15	7.85
Soaked (%) CBR	6.35	6.23	7.05	5.55

Table 3.2: Results of Subgrade Soil (Clay) Test Stabilization with Binding Cementitious Products at Different Percentages and Combination

SAMPLE LOCATION	SOIL + FIBRE RATIO	MDD (kN/m ³)	OMC (%)	UNSOAKED CBR (%)	SOAKED CBR (%)	UCS(kPa)	LL(%)	PL(%)	IP(%)	SIEVE #200	AASHTO/USCS (Classification)	Notes
CLAY SOIL+IRVINGA GARBONESIS FIBRE												
IWOFE ROAD OBIO/AKPO L.G.A	100(%)	1.685	15.28	7.35	6.35	87.85	68.35	37.25	31.10	76.35	A-7-6/CH	POOR
	99.75+0.25(%)	1.645	15.50	10.25	8.89	120	68.05	38.20	29.85	76.35	A-7-6/CH	POOR/GOOD
	99.50+0.50(%)	1.602	15.85	13.35	12.45	175	67.83	38.20	29.63	76.35	A-7-6/CH	GOOD
	99.25+0.75(%)	1.584	16.25	15.25	14.80	225	67.45	38.17	29.28	76.35	A-7-6/CH	GOOD
	99+1.0(%)	1.550	16.60	14.65	13.65	286	66.96	38.03	28.93	76.35	A-7-6/CH	GOOD
CHOKOCHO ROAD ETCHE L.G.A	100(%)	1.635	16.28	7.75	6.23	78.75	53.85	29.30	24.55	80.25	A-7-6/CH	POOR
	99.75+0.25(%)	1.606	16.62	10.85	8.62	94	53.45	28.90	24.15	80.25	A-7-6/CH	POOR/GOOD
	99.50+0.50(%)	1.573	16.98	12.96	10.85	165	53.08	29.28	23.80	80.25	A-7-6/CH	GOOD
	99.25+0.75(%)	1.525	17.24	15.35	13.65	195	52.85	29.21	23.64	80.25	A-7-6/CH	GOOD
	99+1.0(%)	1.493	17.55	13.65	12.40	245	52.40	29.20	23.25	80.25	A-7-6/CH	GOOD
NDONI ROAD OGBA/EGBE MA/NDONI L.G.A	100(%)	1.657	16.05	8.15	7.05	105.75	62.15	31.35	31.05	83.65	A-7-6/CH	POOR
	99.75+0.25(%)	1.618	16.35	11.30	9.35	167	61.80	32.33	31.82	83.65	A-7-6/CH	POOR/GOOD
	99.50+0.50(%)	1.595	16.67	14.25	11.80	198	61.53	32.25	29.55	83.65	A-7-6/CH	GOOD
	99.25+0.75(%)	1.553	16.93	15.35	14.83	234	61.15	32.30	29.23	83.65	A-7-6/CH	GOOD
	99+1.0(%)	1.518	17.28	15.15	13.35	295	58.75	32.30	28.85	83.65	A-7-6/CH	GOOD
OGBELE ROAD AHODA EAST L.G.A	100(%)	1.697	15.73	7.85	5.55	85.35	58.33	26.58	32.17	78.45	A-7-6/CH	POOR
	99.75+0.25(%)	1.665	16.18	10.05	8.20	123	58.08	26.47	31.86	78.45	A-7-6/CH	POOR/GOOD
	99.50+0.50(%)	1.610	16.45	13.73	10.35	185	57.81	26.66	31.42	78.45	A-7-6/CH	GOOD
	99.25+0.75(%)	1.585	16.83	15.85	13.85	222	57.55	26.73	31.08	78.45	A-7-6/CH	GOOD
	99+1.0(%)	1.542	17.05	13.98	12.73	274	57.72	27.62	29.93	78.45	A-7-6/CH	GOOD

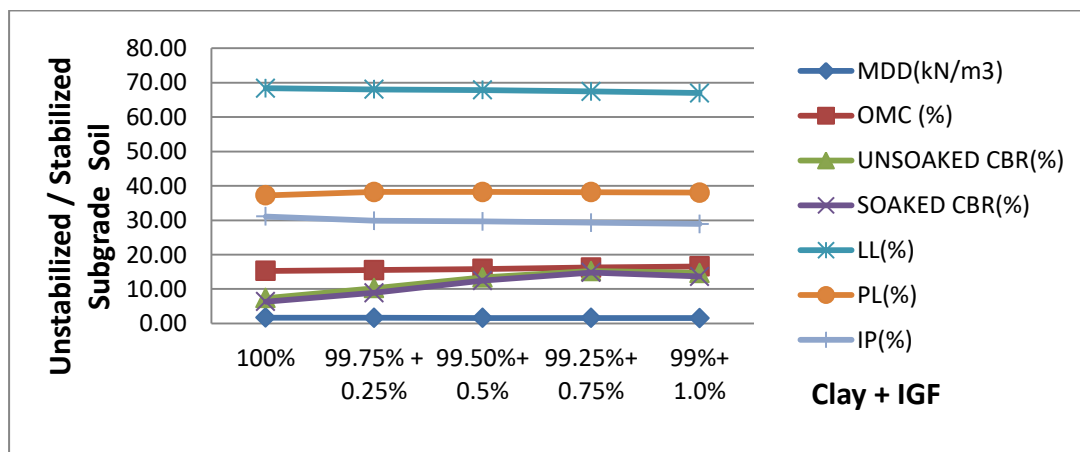


Figure 3.1: Subgrade Stabilization Test of Clay Soil from Iwofe, in Obio/Akpor L.G.A of Rivers State with IGF at Different Percentages and Combination

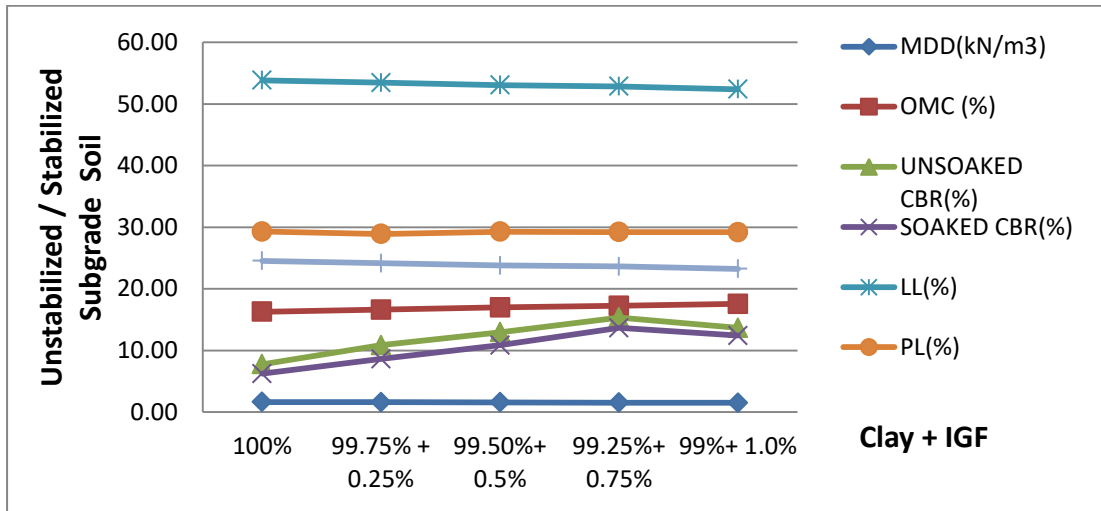


Figure 3.2:Subgrade Stabilization Test of Clay Soil from Chokocho in Etche L.G.A of Rivers State with IGF at Different Percentages and Combination

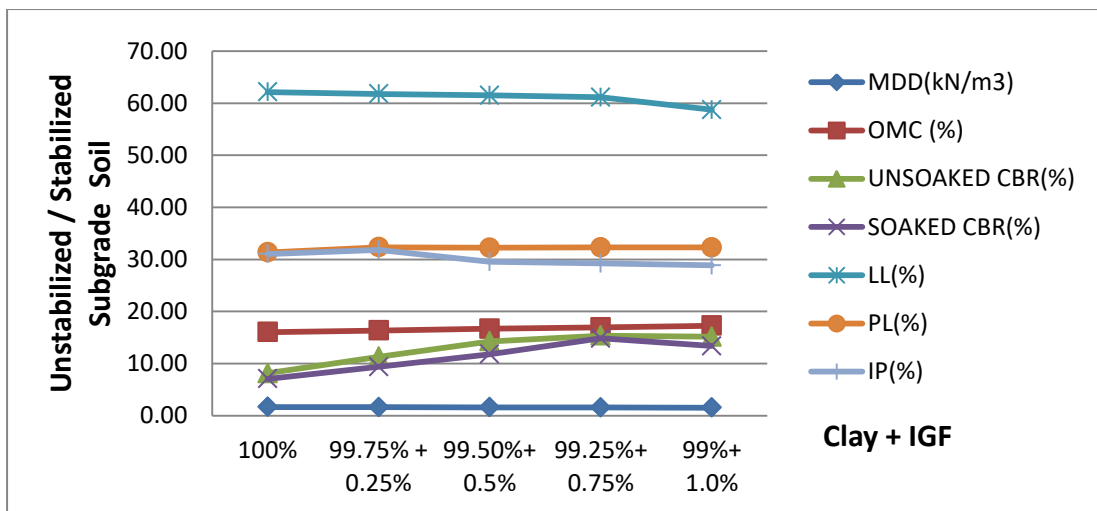


Figure 3.3:Subgrade Stabilization Test of Clay Soil from Ndoni in Ogna/Egbema/Ndoni L.G.A of Rivers State with IGF at Different Percentages and Combination

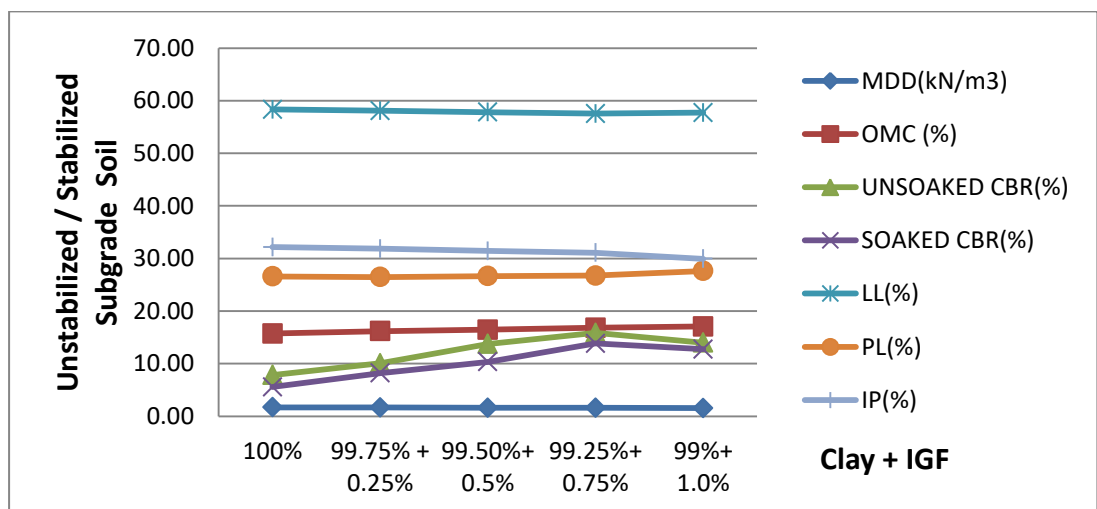


Figure 3.4:Subgrade Stabilization Test of Clay Soil from Ogbele in Ahoada-East L.G.A of Rivers State with IGF at Different Percentages and Combination

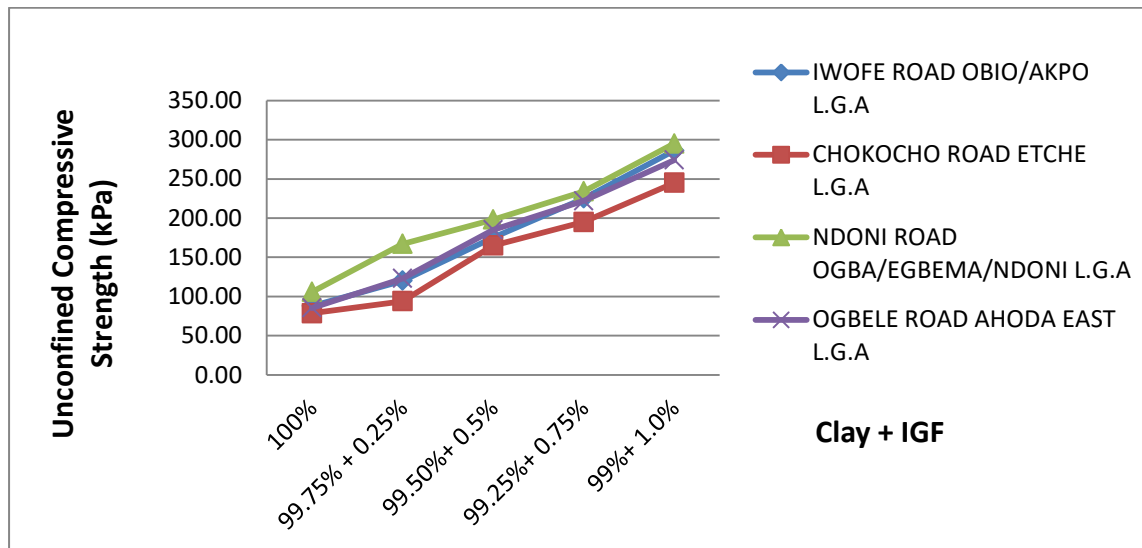


Figure 3.5: Unconfined Compressive Strength (UCS) of Niger Deltaic Clay Soils Subgrade with IGF of (Iwofe, Chokocho, Ndoni, and Ogbele Towns), Rivers State

4. CONCLUSIONS

The following conclusions were made from the experimental research results.

- i. The soils are classified as A – 7 – 6 /CH on the AASHTO classification schemes / Unified Soil Classification System as shown in table 3.1
- ii. The soils are dark grey in colour (from wet to dry states) with plastic index of 31.10%, 24.55%, 31.05%, and 32.17% respectively for Iwofe, Chokocho, Ndoni, and Ogbele Town Roads.
- iii. Results of compaction test determination showed decreased in MDD values while OMC values increased, both parameters adhered to corresponding percentage increase ratios.
- iv. Comparably, results indicated increased in CBR altered soils with optimum mixed percentage ratios of 0.75% to soils.
- v. Comparably, results of UCS stabilized soils increased with percentages ratio variations.

REFERENCES

- [1] Khedari, J., Suttidonk, B., Pratinhong, N., Hirunlabh, J. (2001). New Light Weight Composite Construction Materials with low Thermal Conductivity. *Cement and Concrete Composites*, 23, 65–70.
- [2] Ramakrishna, G. and Sundararajan, T. (2005). Studies on the Durability of Natural Fibres and the Effect of Corroded Fibres on the Strength of Mortar. *Cement and Concrete Composites*, 27(5): 575-582
- [3] Charles, K., Essien, U., Gbinu, S. K. (2018). Stabilization of Deltaic Soils using Costus Afer Bagasse Fiber. *International Journal of Civil and Structural Engineering Research*. 6(1): 148-156
- [4] Ghavami, Kh., Toledo Filho, R.D., and Barbosa, N.P., (1999). Behavior of Composite Soil Reinforced with Natural Fibres. *Cement and Concrete Composites*, 21, 39–48
- [5] Prabakar, J. and Sridhar, R.S. (2002). Effect of Random Inclusion of Sisal Fibre on Strength Behavior of Soil. *Construction and Building Materials*, 16, 123–131
- [6] Bouhicha, M., Aouissi, F. and Kenai, S. (2005). Performance of Composite soil reinforced with Barley Straw. *Cement and Concrete Composites* 27(5): 617–621
- [7] Ola, S. A. (1974). Need for Estimated Cement Requirements for Stabilizing Lateritic soils. *Journal of Transportation Engineering, ASCE*, 100(2):379–388.
- [8] Allam, M. M. and Sridharan, A. (1981). Effect of Repeated Wetting and Drying on shear Strength. *Journal of Geotechnical Engineering, ASCE*, 107(4):421–438
- [9] Omotosho, P. O. (1993). Multi-Cyclic Influence on Standard Laboratory Compaction of Residual Soils. *Engineering Geology*. 36, 109–115.
- [10] Omotosho, P. O. and Akinmusuru, J. O. (1992). Behaviour of soils (lateritic) Subjected to Multi-Cyclic Compaction. *Engineering Geology*, 32, 53–58